

April 9, 2000

Larry Hand  
Fieldbrook, CA 95519

**Subject: Leachfield Suitability Report, APN 525-072-02, Fieldbrook**

**INTRODUCTION**

This report presents the results of my field and laboratory investigation for on site sewage disposal at the referenced site. The site is located south of Fieldbrook Road, and slightly west of Rock Pit Road. Figure One shows the location of monitoring wells, hand auger holes, hand dug pits, percolation test holes, approximate building site, general topography, existing fences, and the proposed sewage disposal system.

The proposed residence will have three bedrooms. Water will be provided by a community water system. The adjoining lots are served by a community water system.

**SCOPE OF WORK and METHODS**

My scope of work included: determining a suitable area for on site sewage disposal; describing and sampling soils; installing groundwater monitoring wells; monitoring the groundwater table during the wet weather period; performing percolation testing; performing laboratory textural analyses (by hydrometer); providing a proposed system design and layout; and preparing this report.

Soil descriptions follow the guidelines of the USDA soil classification system. I collected samples at selected horizons. Laboratory results are presented on the soil logs. The numbers of the groundwater monitoring wells, and percolation test holes coincides with the number of the soil logs. The monitoring wells were placed in hand auger holes. The percolation test pits were excavated by hand.

Appendix I is the soil logs. Appendix II is a plot of the texture of site soils. Appendix III is the results of percolation testing. Appendix IV is the results of groundwater monitoring. Appendix V contains details of groundwater monitoring wells. Appendix VI is a calculation of the linear feet of leachline trench plus a typical trench cross section.

**DESCRIPTION OF SITE**

The slope gradient is approximately 3 percent in the proposed absorption area. Vegetation consists of annual grasses. There is an ephemeral creek (Figure One) that is a consequence of water redirected through a culvert under Fieldbrook Road. This creek is greater than 50 feet from the proposed absorption area.

## DESCRIPTION OF SITE SOILS AND GROUNDWATER

The soil profile consists of approximately one foot of Silt Loam topsoil over a Silty Clay Loam to Clay Loam. The Color of the subsoil is dark yellowish brown. Distinct mottles are present below a depth of four feet. Structure is well developed granular in the topsoil and moderately developed medium subangular blocky in the subsoil.

During the time of my investigation, the water table rose to 3.5 feet below the ground surface, in Monitor Well Numbers 2 and 3 (Appendix IV).

## PERCOLATION TESTING AND TEXTURE

The results of percolation testing ranged from 12-24 minutes per inch (Appendix III).

The texture of site soils falls within zone 3-4 (marginal) of the Humboldt County Soil Percolation Suitability Chart (Appendix II).

## CONCLUSION

Due to the seasonally high groundwater table, a low-pressure distribution network, or pressure dosing system, should be installed on this parcel. The proposed pressure dosing system utilizes one and one-half foot deep trenches with one foot of washed pea gravel beneath the distribution pipe (Appendix VI). Effluent is applied under pressure in a series of doses, allowing for uniform distribution and helping to maintain aerobic conditions in the soil.

The total length of distribution line should be 225 feet. I recommend using 5, 45-foot lines, spaced 6 foot on center. This requires an area 24 feet by 45 feet. The replacement area (reserve field) should be of equal area.

## GENERAL NOTES

The recommended depth, width, length, and spacing of distribution line trenches are specified in Appendix VI. Gravel should be washed pea gravel. Filter fabric should cover the gravel filled trenches. The filter fabric should be covered by topsoil-based fill.

Force main, or supply line trenches, should be 2.0 feet deep and backfilled with compacted native material.

All trenches for distribution lines should be along contour. Attempt to achieve a zero slope gradient for the base of trenches and distribution pipes.

All trench sidewalls should be raked, or plucked, of smeared surfaces.

Supply lines should be 2-inch diameter pipe. Distribution lines should be 1 ¼ inch diameter pipe. All piping and fittings should be schedule 40 PVC. Irregular bends may be constructed of flexible spa hose, electrical PVC, or equivalent.

A 5/32-inch diameter hole should be drilled in the distribution lines every 3 feet. No holes should be closer than 2 feet from the end of the line. For a 45-foot line length, this spacing allows for 14 holes per line. All burrs from drilling should be removed. Holes should be drilled in a line and placed down in the trench.

A gate valve approved for sewage transmission should be placed at the proximal end of each distribution line. These gate valves will be used to regulate pressure head. At the terminal end of each distribution line, pressure head should be 3 feet. The valves should be placed in a christy box or similar

The end of each distribution line should be equipped with a removable, capped turn-up that provides for above ground access for cleaning or flushing. Using a long sweep elbow will make clean out easier. The capped end should be brought up above or flush with grade. As the trench is filled, the turn-up may be placed inside a short length of 4" or 6" terra cotta or PVC to protect it from lawn mowing damage while still providing easy access.

The pressure head is the height liquid will rise above the turn-up elbow when the pump is running. To measure the pressure head, glue a four-foot length of pipe (preferable clear) to a threaded adapter that will screw onto the turn-up adapters. Replace the turn-up cap with the pipe and adapter. Turn on the pump and adjust the gate valve until the level of water reaches the desired height.

The pump should be capable of supplying the calculated flow rate (Appendix VI) against the total dynamic head (Appendix VI).

The float controls should be set to the dosing volume specified in Appendix VI.

The septic tank should have a minimum volume of 1800 gallons with an effluent filter installed at the outlet of the tank. Alternatively, you can use two, 750-gallon tanks in series. It should be water tight and heavy enough not to float when empty. A watertight access riser and lid, set at grade, should provide access to each compartment. The top of the tank should be covered by minimum of 6 inches of fill. The tank should be installed level. The slope of the sewer line to the septic tank should be uniform and not less than 2 percent. Install a cleanout at the junction of the septic tank with the building.

The pumping tank should be 1200 gallons. It should be watertight. A watertight riser and lid, set at grade, should provide access. The top of the tank should be covered by minimum of 6 inches of fill.

The pumping tank should be equipped with a visual and audible high water alarm on a separate electrical circuit as the pump. The alarm should include a float switch, that when tripped will turn on a light and set off a buzzer. The light should be placed inside the residence. The buzzer should be outside.

The location of septic tanks should be greater than 5 feet from property lines and 10 feet from buildings.

Only quality materials should be used, and installed in accordance with the manufacturers recommendations.

All electrical work should be performed in conformance with applicable codes and standards. All switches, relays, and NEMA approved closures shall be water proof, corrosion proof resistant, and explosion proof. All electrical connections should be made outside of the tank. Power cords should be plugged into a NEMA approved outdoor receptacle mounted outside of the tank. Electrical connections can be made inside the tank only if wired inside a sealed, watertight box.

During construction, the contractor should use caution to minimize disruption and compaction of native soils. Removal or compaction of the topsoil may destroy the suitability of the site.

This system must be installed during dry weather. No site preparation or low-pressure pipe construction work should occur if the soil is wet.

#### CLOSURE

The system will function properly only if is installed using high quality workmanship and materials, and maintained regularly. I do not warrant the operation or proper functioning of the system for any period of time. At their discretion, Humboldt County Department of Environmental Health (HCDEH) will grant final approval of the suitability of the information provided in this report and any required addenda. In addition, through inspections, HCDEH will determine the adequacy of construction at build-out.

Thank you for requesting my services. If you have any questions, please call.

Sincerely,

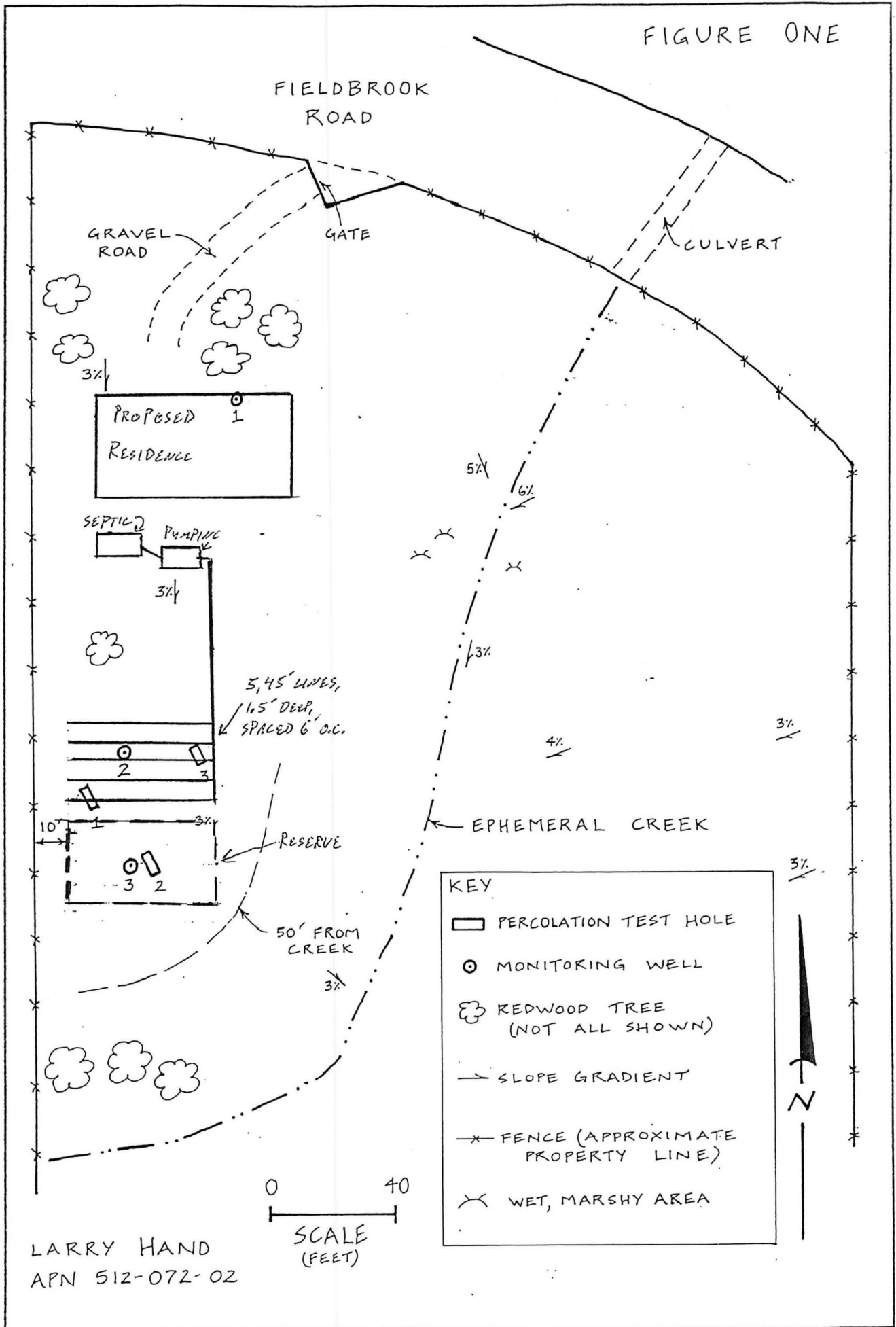


Mark Verhey, Registered Geologist  
R.G. 6,729 Expires 1/31/01

#### References Cited

Cogger, C., Carlile, B., Osborne, D, 1982. Design and installation of low-pressure pipe waste treatment systems, North Carolina State Univ., UNC Sea Grant Publication UNC-SG-82-03, 31 pp.

FIGURE ONE

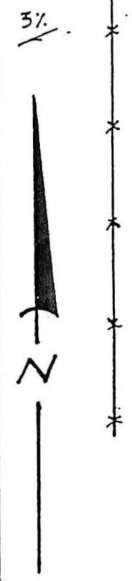


**KEY**

- ▭ PERCOLATION TEST HOLE
- MONITORING WELL
- ☁ REDWOOD TREE (NOT ALL SHOWN)
- ↘ SLOPE GRADIENT
- x- FENCE (APPROXIMATE PROPERTY LINE)
- ⊗ WET, MARSHY AREA

0 40  
SCALE (FEET)

LARRY HAND  
APN 512-072-02



SOIL LOG

APPENDIX I

Job Name Larry Hand  
Equipment hand auger

Date 6-Dec-98

Log No. 2
By MV

Laboratory Data				depth (feet)	USDA Soil Classification texture, consistency, moisture, color, symbol misc: structure, roots, pores, clay films
% coarse frag's	% sand/clay/silt	% water	dry density (gm/cc)		
2%	24.5/36.6/38.9	22.3	1.55	-	Loam, very friable, damp, black, (L). moderate medium granular structure, many fine roots, few medium roots.
				1	
				-	
				-	
				2	
				-	
				-	
				3	
				-	
				-	
4	Clay Loam, medium stiff, damp, dark yellowish brown and grey with many distinct mottles, (CL). moderate medium subangular blocky				
-					
-					
5					
-					
-					
6					
-					
-					
-					
7	Sandy Clay Loam, medium stiff, damp, dark yellowish brown and grey with many distinct mottles, (SCL).				
-					
-					
-					
8					
-					
-					
-					
-					
-					
9	becomes gravelly  Bottom of hole, refusal on rock (9.0 ft).				
-					
-					
-					
10					

# SOIL LOG

# APPENDIX I

**Job Name** Larry Hand  
**Equipment** hand auger

**Date** 6-Dec-98

<b>Log No.</b> 3
<b>By</b> MV

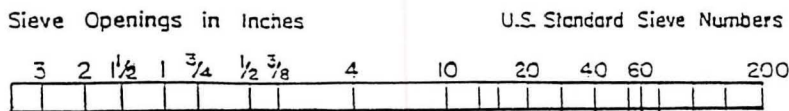
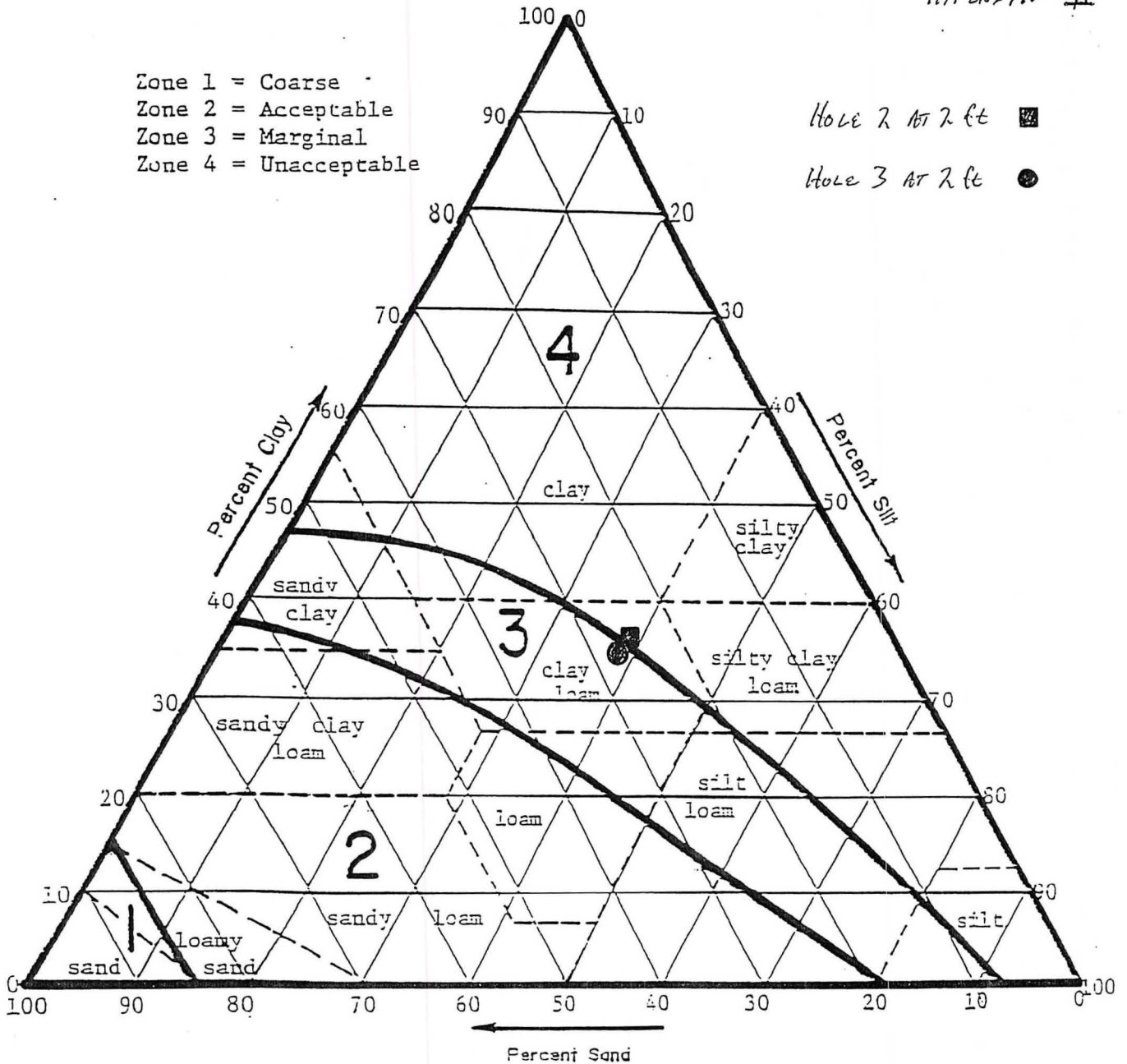
Laboratory Data				depth (feet)	USDA Soil Classification texture, consistency, moisture, color, symbol misc: structure, roots, pores, clay films	
% coarse frag's	% sand/clay/silt	% water	dry density (gm/cc)			
3%	26.4/35.1/38.5	25.4	1.47	-	Loam, very friable, damp, black, (L). moderate medium granular structure, many fine roots, few medium roots.	
				1		
				-		
				-		
				2		Clay Loam, medium stiff, damp, dark yellowish brown and grey with few faint mottles, (CL). moderate medium subangular blocky, few medium roots,
				-		
				-		
				3		
				-		
				-		
4	Sandy Clay Loam, medium stiff, damp, dark yellowish brown and grey with few distinct mottles, (SCL). moderate medium subangular blocky  becomes many distinct mottles					
-						
-						
5						
-						
-						
6						
-						
-						
7						
-						
-						
8						
-						
-	Bottom of hole in same (8.7 ft)					
9						
-						
-						
10						

SOIL PERCOLATION SUITABILITY CHART

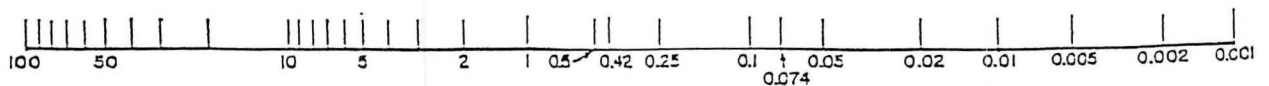
APPENDIX II

- Zone 1 = Coarse
- Zone 2 = Acceptable
- Zone 3 = Marginal
- Zone 4 = Unacceptable

Hole 2 at 2 ft   
 Hole 3 at 2 ft



USDA	GRAVEL	SAND					SILT	CLAY
		Very Coarse	Coarse	Medium	Fine	Very Fine		



- Plot texture on triangle based on percent sand, silt, and clay as determined by hydrometer analysis.
- Adjust for coarse fragments by moving the plotted point in the sand direction an additional 2% for each 10% (by volume) of fragments greater than 2mm in diameter.
- Adjust for compactness of soil by moving the plotted point in the clay direction an additional 15% for soils having a bulk density greater than 1.7 g/cc.



Percolation Test DataJob Name Larry HandLocation FieldbrookAPN 502-072-02Testing Period Wet WeatherWater Supply CommunityTest No. 1Date: 30-Jan-99By: MV

Hole Data		
depth	method	size
0 -18 in	hand tools	12 in diam

Presoak Data			
Start time and date	9:00	30-Jan-99	
End time and date	10:24	30-Jan-99	

Test Data			
	time	elapsed time (min.)	drop (in.)
start	10:24		
read	10:39	15	3/4
	10:40		
	10:55	15	3/4
	10:56		
	11:11	15	3/4
	11:11		
	11:26	15	5/8
	11:26		
	11:41	15	5/8
	11:41		
	11:56	15	5/8
	11:56		
	12:11	15	5/8

STABILIZED PERCOLATION RATE = 24 minutes/inch

Note: See Appendix I for soil log.

Percolation Test DataJob Name Larry HandLocation FieldbrookAPN 502-072-02Testing Period Wet WeatherWater Supply CommunityTest No. 2Date: 30-Jan-99By: MV

Hole Data		
depth	method	size
0 -18 in	hand tools	12 in diam

Presoak Data			
Start time and date	9:00	30-Jan-99	
End time and date	10:24	30-Jan-99	

Test Data			
	time	elapsed time (min.)	drop (in.)
start	10:22		
read	10:37	15	1 3/4
	10:38		
	10:53	15	1 1/2
	10:54		
	11:09	15	1 1/3
	11:09		
	11:24	15	1 1/2
	11:24		
	11:39	15	1 1/2
	11:39		
	11:54	15	1 1/3
	11:54		
	12:09	15	1
	12:09		
	12:24	15	1

STABILIZED PERCOLATION RATE = 15 minutes/inch

Note: See Appendix I for soil log.

Mark Verhey, Registered Geologist

Percolation Test Data

Job Name Larry Hand  
 Location Fieldbrook  
 APN 502-072-02  
 Testing Period Wet Weather  
 Water Supply Community

Test No.	3
----------	---

Date: 30-Jan-99  
 By: MV

Hole Data		
depth	method	size
0 -18 in	hand tools	12 in diam

Presoak Data			
Start time and date	9:00	30-Jan-99	
End time and date	10:24	30-Jan-99	

Test Data			
	time	elapsed time (min.)	drop time (in.)
start	10:23		
read	10:38	15	1 3/4
	10:39		
	10:54	15	1 3/4
	10:55		
	11:10	15	1 5/8
	11:10		
	11:25	15	1 1/2
	11:25		
	11:40	15	1 1/2
	11:40		
	11:55	15	1 1/4
	11:55		
	12:10	15	1 1/4
	12:10		
	12:25	15	1 1/4

STABILIZED PERCOLATION RATE = 12 minutes/inch

Note: See Appendix I for soil log.

Mark Verhey, Registered Geologist

Groundwater Monitoring Data

Appendix IV

**Hole#**

**2**

Stickup Height  
1.0 foot

Date	Reading from top of pipe (ft)	water depth below ground (ft)	24 hour Rainfall	Rainfall To Date
12/12/98	6.08	5.08	0.07	21.02
12/28/98	7.33	6.33	0.04	22.49
1/23/99	4.50	3.50	0.42	26.70
3/25/99	4.83	3.83	2.41	44.50
3/26/99	6.08	5.08	0.03	44.53
3/27/99	7.08	6.08	0.00	44.53
3/28/99	7.75	6.75	0.01	44.54
3/30/99	5.83	4.83	1.05	46.13
3/31/99	5.00	4.00	0.30	46.43
4/1/99	6.00	5.00	0.02	46.45
4/2/99	6.67	5.67	0.09	46.54
4/3/99	7.17	6.17	0.07	46.61
4/4/99	7.83	6.83	0.01	46.62
4/5/99	7.17	6.17	0.34	46.96
4/6/99	7.50	6.50	0.00	46.96
4/7/99	7.75	6.75	0.28	47.24
4/8/99	6.92	5.92	0.47	47.71
4/9/99	6.92	5.92	0.07	47.78
4/11/99	6.92	5.92	0.43	48.21
4/12/99	7.67	6.67	0.00	48.21
4/13/99	9.17	8.17	0.00	48.21

**Hole#**

**3**

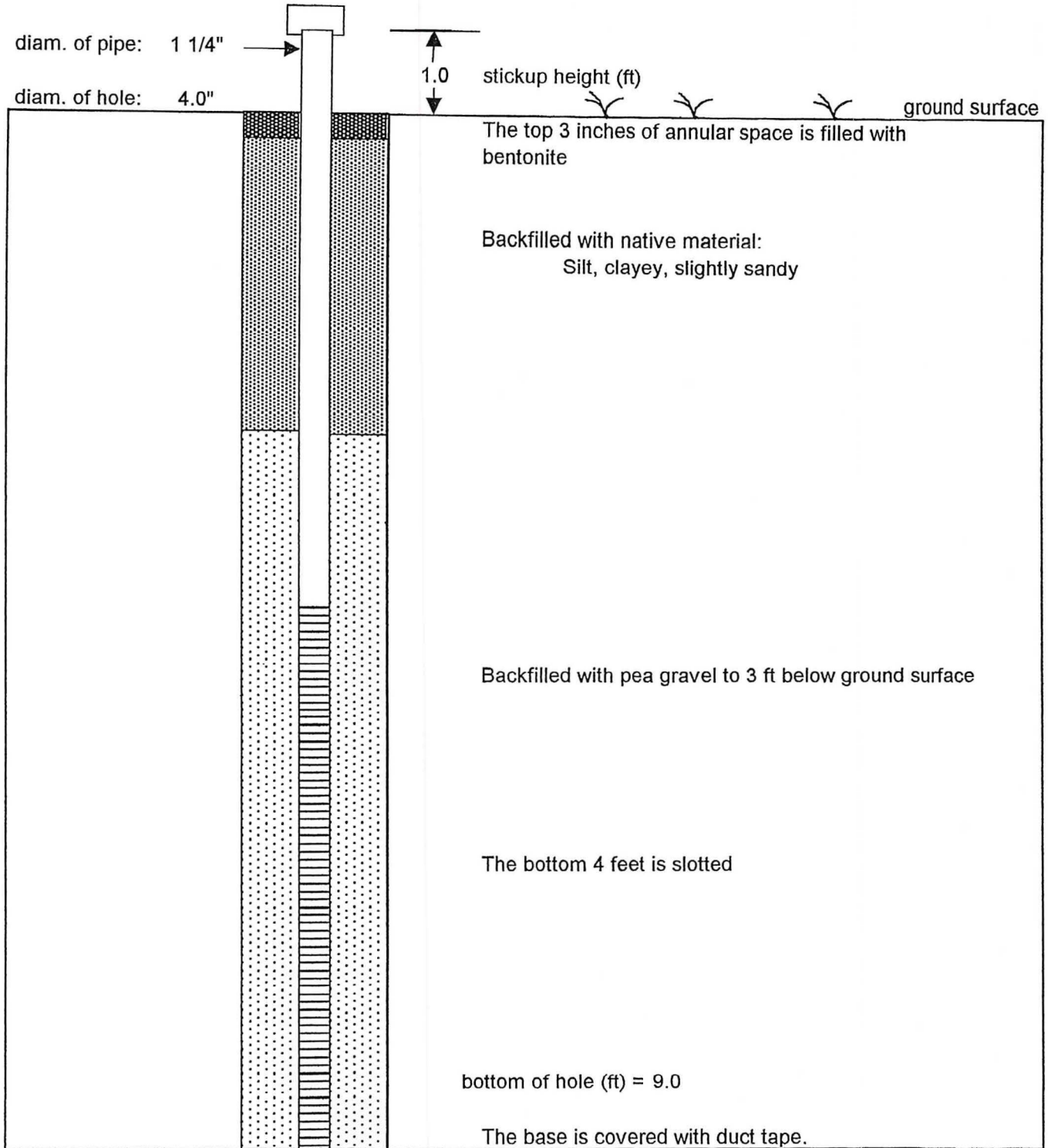
Stickup Height  
1.3 feet

Date	Reading from top of pipe (ft)	water depth below ground (ft)	24 hour Rainfall	Rainfall To Date
12/12/98	7.25	5.95	0.07	21.02
12/28/98	8.00	6.70	0.04	22.49
1/23/99	4.80	3.50	0.42	26.70
3/25/99	5.33	4.03	2.41	44.50
3/26/99	5.33	4.03	0.03	44.53
3/27/99	6.67	5.37	0.00	44.53
3/28/99	7.50	6.20	0.01	44.54
3/30/99	7.75	6.45	1.05	46.13
3/31/99	6.25	4.95	0.30	46.43
4/1/99	5.25	3.95	0.02	46.45
4/2/99	6.67	5.37	0.09	46.54
4/3/99	7.25	5.95	0.07	46.61
4/4/99	7.75	6.45	0.01	46.62
4/5/99	8.00	6.70	0.34	46.96
4/6/99	7.67	6.37	0.00	46.96
4/7/99	7.17	5.87	0.28	47.24
4/8/99	8.00	6.70	0.47	47.71
4/9/99	7.00	5.70	0.07	47.78
4/11/99	7.17	5.87	0.43	48.21
4/12/99	7.17	5.87	0.00	48.21
4/13/99	7.92	6.62	0.00	48.21

Groundwater Monitoring Well Installation Details

Job Name Larry Hand  
 Location Fieldbrook  
 APN 502-072-02

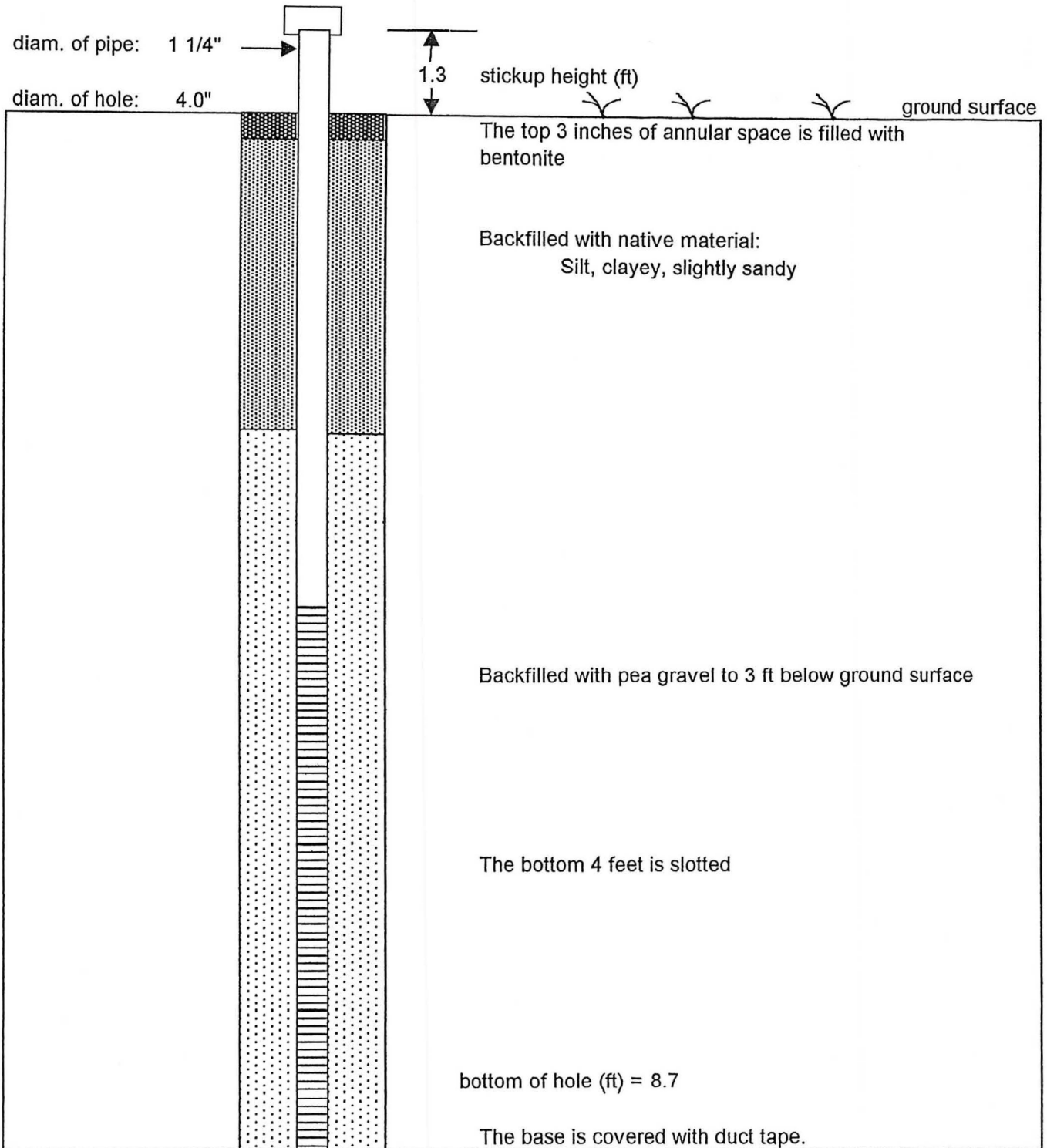
Hole #	<b>2</b>
Installed By	<u>MAV</u>
Length Scored	<u>bottom 4 ft</u>
Total length of Pipe (ft)	<u>10</u>
Stickup Height (ft)	<u>1.0</u>



Groundwater Monitoring Well Installation Details

Job Name Larry Hand  
 Location Fieldbrook  
 APN 502-072-02

Hole #	<b>3</b>
Installed By	<u>MAV</u>
Length Scored	<u>bottom 4 ft</u>
Total length of Pipe (ft)	<u>10</u>
Stickup Height (ft)	<u>1.3</u>



**Design Data**

Estimated Daily Sewage Flow: 450 gallons per day (3 BR residence)  
 Texture: Clay Loam (Appendix II)  
 Results of Percolation Testing: 24 minutes per inch (mpi)  
 Depth to High Groundwater: 3.5 ft (see Appendix III)

**Design Details**

**A) Loading Rate**

Use a loading rate of 0.4 gallons per day per square foot (gpd/sq ft)

**B) Total length of distribution line = daily sewage flow / loading rate / 5 Use 5, 45 foot**

Use a line spacing of 6 foot on center lines spaced 6 foot on center.  
 =  $450 / 0.4 / 5 = 225$  feet This uses an area 45' x 24'.

**C) Distribution Network**

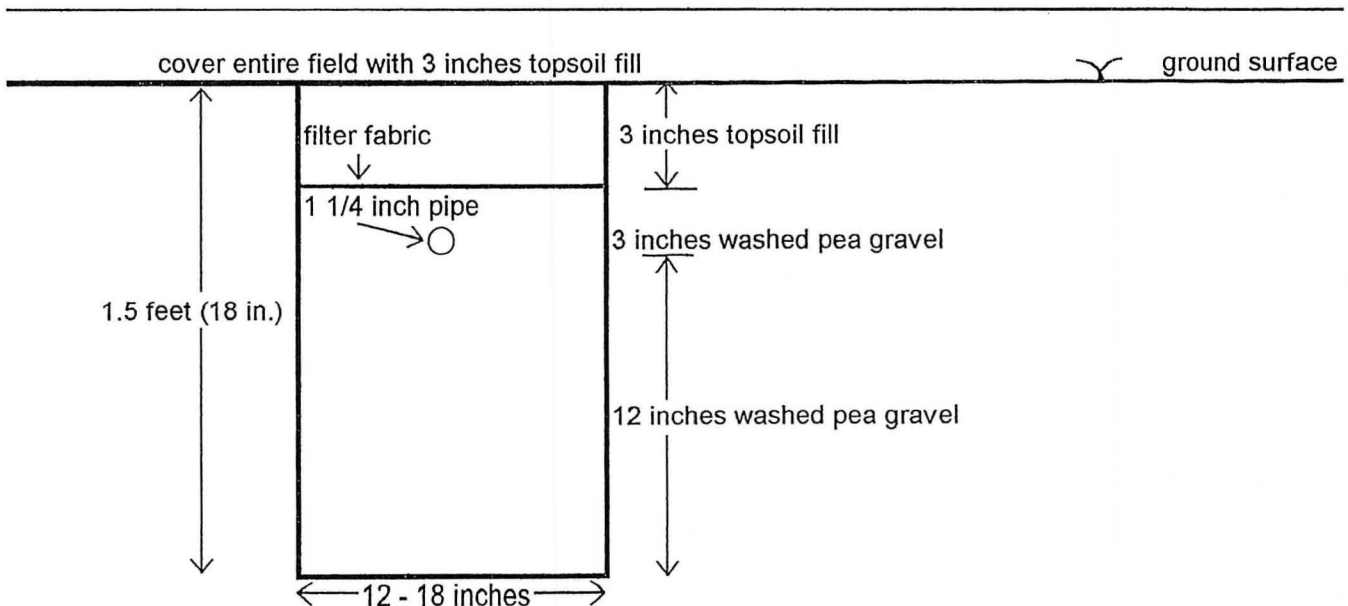
The pressure head at the terminal end of the line should be 3 feet. Pressure head should be regulated by a dedicated ball or gate valve(s) approved for sewage transmission. Holes should be drilled 5/32" diameter placed at 3 intervals. No holes should be closer than 2' from the terminal end of the line. This spacing and recommended line length allows for 14 holes per line. With 3 feet of head and a 5/32" diameter hole, the flow rate is 0.5 gallons per minute per hole (gpm/hole). With 14 holes per line, 0.5 gpm/hole, and 5 lines, the system flow rate is **35 gpm**

**D) Pump Selection and Recommended Dosing Volume**

The pump must have enough power to pump effluent at the calculated flow rate against the total head (resistance) encountered in the distribution system. A 1/2 horse power pump will be adequate. The on/off float switches should be set to supply 80 gallons per dose.

**TYPICAL TRENCH DETAIL**

Width: 12-18 inches	Rock under pipe: 12 inches of washed pea gravel
Depth: 1.5 feet	Rock over pipe: 2 inches of washed pea gravel
Slope: max. of 1.5 inches per 50 feet	Material over rock: filter fabric
	Material over fabric: topsoil fill





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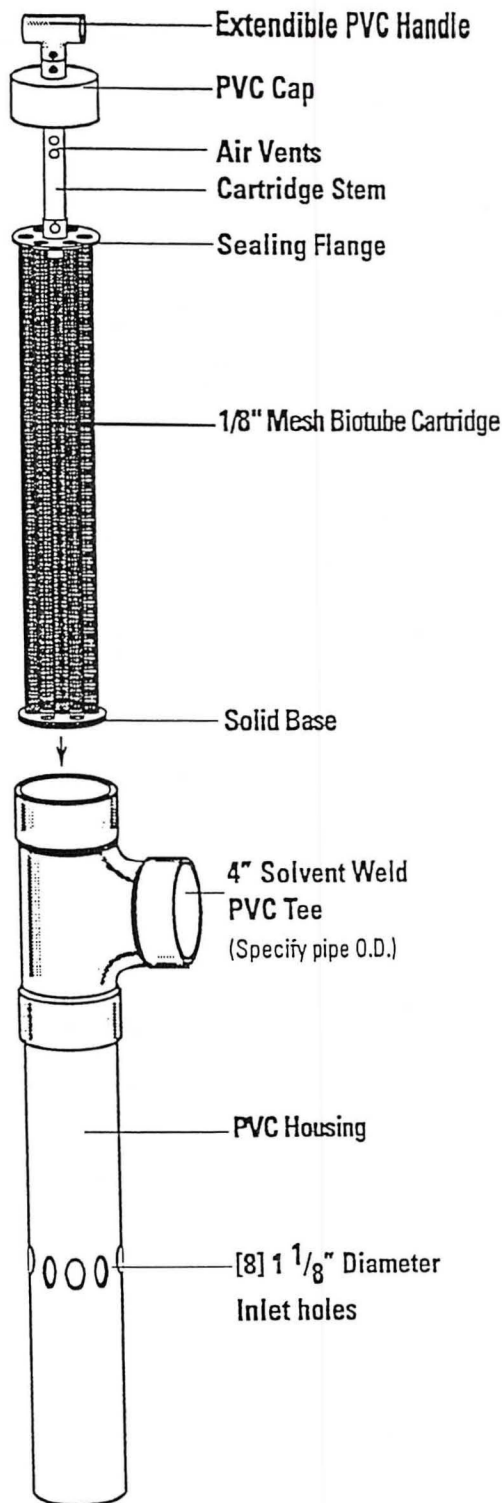
(541) 459-4449

FACSIMILE:

(541) 459-2884

# OSI Biotube<sup>®</sup> Effluent Filter

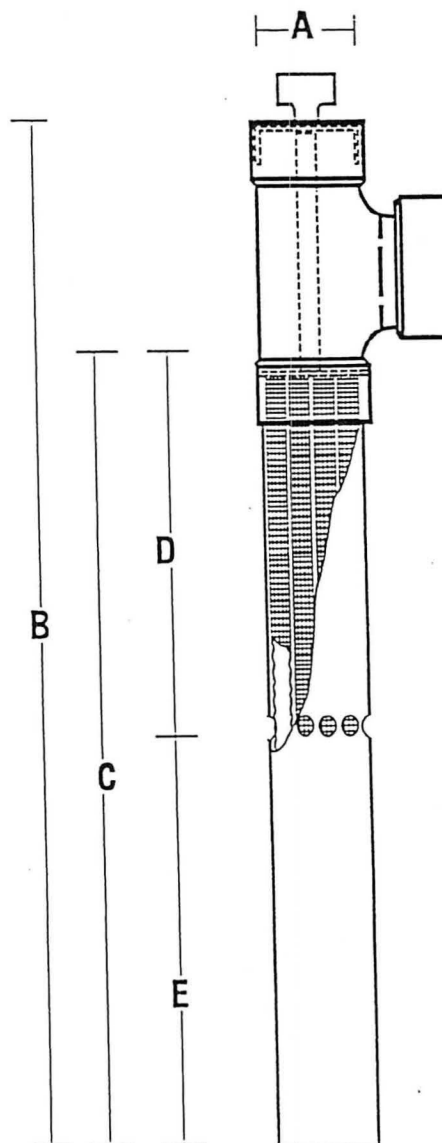
# Specifications



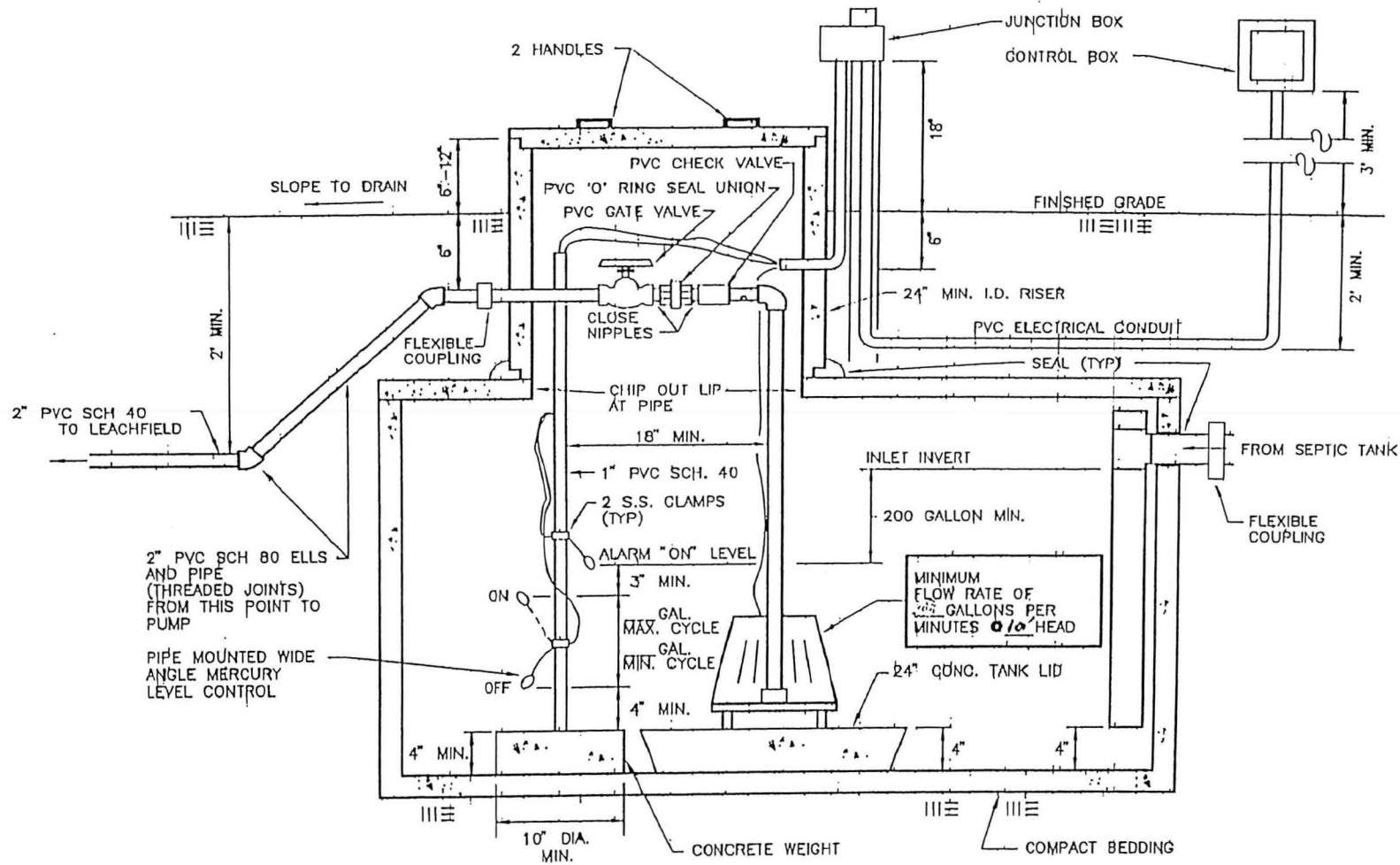
## Dimensions

Model	FT0444-36	FT0436-28
A Nominal Diameter	4"	4"
B Vault Height	44"	36"
C Invert Height	38"	30"
D Invert to Inlet Height	16"	11"
E Inlet Hole Height	22"	19"
F Filter Area (ft <sup>2</sup> )	5.3	4.1

Custom sizes available. Call for assistance.

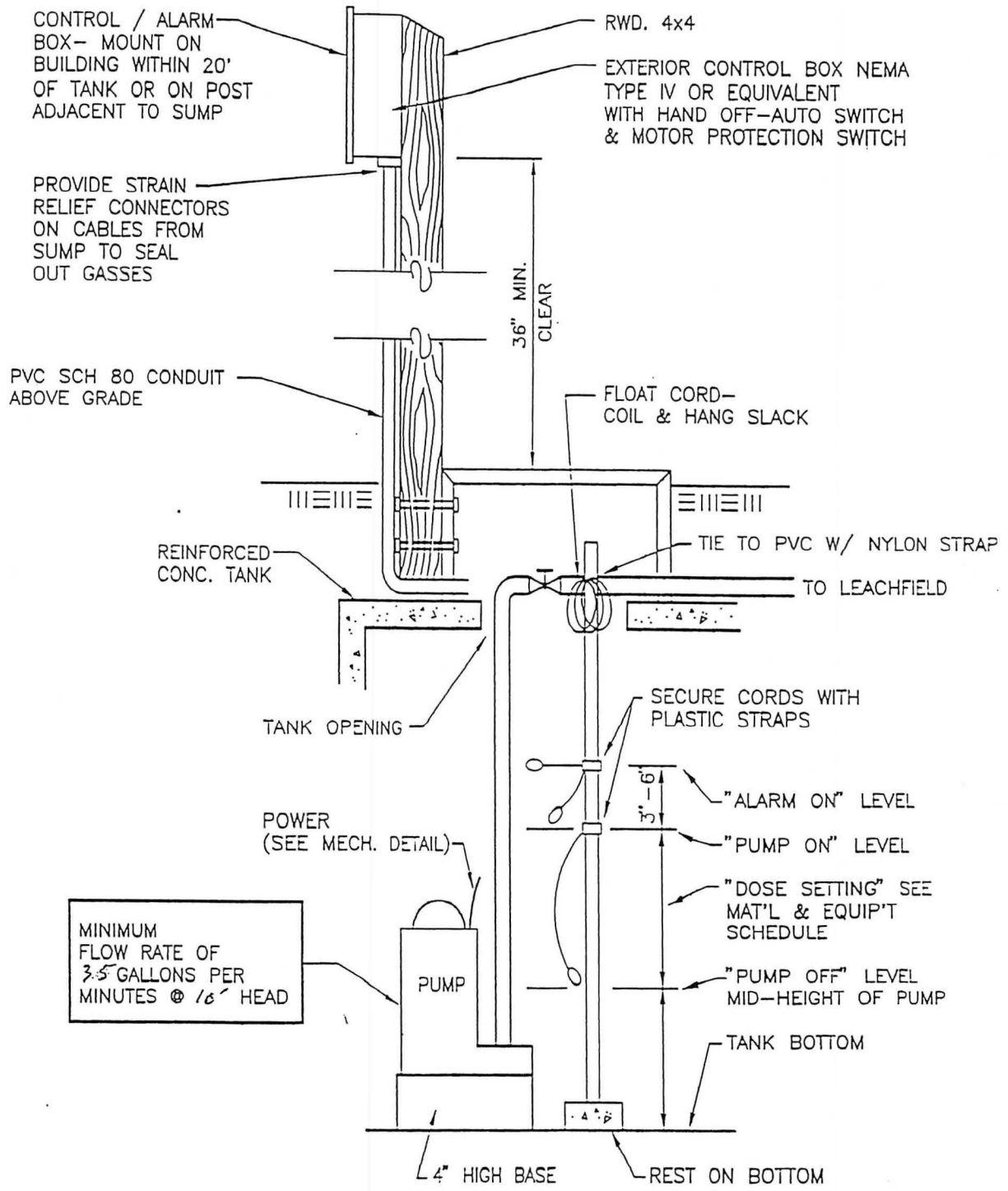






SUMP / PUMP

N.T.S.



CONTROL / ALARM

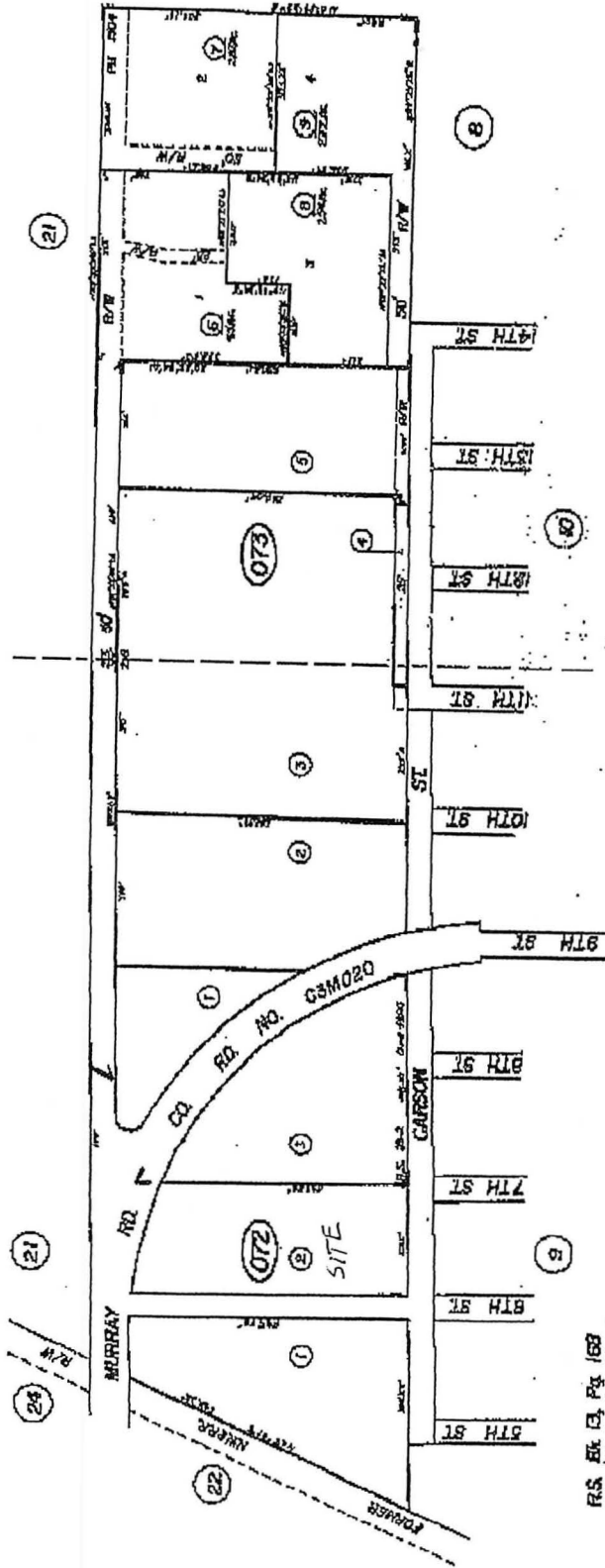
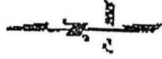
N.T.S.

512-07

FOR SEC. 26, T.7N, R.1E, H.B.M.

NOTES ON TOWN OF FREDERICK:

- RECORDED DATA IS CONTAINED IN R.S. BK. 3, PGS. 63, 64, 65
- AN AMENDED MAP (MURRAY SURVEY NO. 125) SHOWS ADDITIONAL TRACTS & STREETS. THIS MAP WAS NOT RECORDED.
- STREETS HAVE BEEN ADVANCED SUPERVISOR'S ORDINANCES BK. 14, PG. 28 & BK. 15, PG. 3, WITH REFERENCE TO BOTH RECORDS & UNRECORDED SURVEYS.
- 6TH, 8TH, 7TH, 8TH, 10TH, 12TH, 13TH STS. ARE NOT OPEN ON GRADON.



- R.S. BK. 13, PG. 168
- R.S. BK. 20, PG. 51
- R.S. BK. 23, PG. 2
- R.S. BK. 30, PG. 12
- R.S. BK. 29, PG. 63
- CM. NO. 1304 of PM. BK. 11, PG. 104

Assessor's Maps BK. 512 - Pg. 07  
County of Hancock, Me.